Program: BE Civil Engineering

Curriculum Scheme: Revised 2016

Examination: Third Year SemesterVI

Course Code: CEC601and Course Name: GEOTECHNICAL ENGINEERING-II

Time: 1hour Max. Marks: 50

Note to the students:- All the Questions are compulsory and carry equal marks .

Q1.	The coefficient of compressibility is the ratio of
Option A:	Change in void ratio to change in effective stress
Option B:	Volumetric strain to change in effective stress
Option C:	Change in thickness to change in effective stress
Option D:	Stress to strain
Q2.	Under load, the void ratio of a submerged saturated clay decreases from 1.0 to
	0.92. the ultimate settlement of a layer 2 m thick layer will be
Option A:	2 cm
Option B:	4 cm
Option C:	16 cm
Option D:	8 cm
Q3.	When the primary consolidation process in a soil is complete. Then
Option A:	The hydrostatic pressure will become zero
Option B:	The excess pore water pressure will become zero
Option C:	Both the hydrostatic and excess pore water pressure will become zero
Option D:	The effective stress will become zero
Q4.	The relation between undrained cohesion Cu and unconfined compressive
	strength (qu) for cohesive soil is
Option A:	$C_u = 2 q_u$
Option B:	$C_u = (q_u)/2$
Option C:	$C_u = 3q_u$
Option D:	$C_u = (q_u)/3$
Q5.	If the angle of shearing resistance is 30°, the angle which the failure plane makes
	with the major principal plane is
Option A:	15°
Option B:	30°
Option C:	45°

Option D:	60°
Q6.	If 600 kN/m^2 is the undisturbed shear strength of a soil and 300 kN/m^2 is it's
	remoulded shear strength, then sensitivity is
Option A:	0.5
Option B:	1
Option C:	1.5
Option D:	2
Q7.	if an infinite slope of clay at a depth 5 m has cohesion of 1 t/m ² and unit weight
	of 2 t/m ² then the stability number is
Option A:	0.1
Option B:	0.2
Option C:	0.3
Option D:	0.4
Q8.	The method of the slices is applicable to
Option A:	Homogenous soil
Option B:	Stratified soils
Option C:	Non Homogeneous soils
Option D:	Non uniform slopes
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Q9.	In friction circle method of slope stability analysis . if r defines the radius of slip
	circle , the radius of friction circle is
Option A:	r sin φ
Option B:	r
Option C:	r cos φ
Option D:	r tan φ
Q10.	The ratio of horizontal and vertical principal effective stresses when a retaining
	wall is forced against a soil mass is called as
Option A:	Active Earth pressure
Option B:	Passive Earth pressure
Option C:	Pore Pressure
Option D:	Swelling Pressure
Q11.	A rigid retaining wall having the backfill consisting of cohesion less soil having φ
	= 30°. What will be the coefficient of earth pressure at rest?
Option A:	0.8
Option B:	0.5
Option C:	0.4
Option D:	0.6
Q12.	If the movement of failure wedge is upward then the pressure is acting on the
	soil is
Option A:	Active Earth pressure

Option C: Pore Pressure Q13. Cohesion in soil Option A: Decreases active pressure and increases passive resistance Option B: Increases both active pressure and passive resistance Option C: Increases both active pressure and passive resistance Option D: Increases both active pressure and passive resistance Option D: Increases both active pressure and passive resistance Option C: Increases both active pressure and passive resistance Q14. Earth pressure for retaining walls, of less than 6m are obtained by Option A: Analytical method Option B: Considering approximate value Option C: Graphical method Option D: Analytical and Graphical method Q15. Pressure distribution from wedge theory assumption is Option A: Distributed Option A: Distributed Option C: Planar Option D: Hydrostatic Q16. A shallow foundation is usually defined as a foundation which has Option A: Depth less than 0.6m Option A: Depth less than 0.6m Option B: Depth less than or equal to its width Option C: Depth less than 1m Option D: Depth greater than width Q17. The value of shape factor Sc, Sq and Sy for circular base is Option B: 1.3, 1.2, 0.8 Option B: 1.3, 1.2, 0.8 Option B: 1.3, 1.2, 0.5 Option C: 1,0, 1.0, 1.0 Option D: 1.2, 1.0, 1.3 The standard penetration resistance is determined at a number of selected points at interval of Option A: 80 cm Option D: 10 cm Q19. In general shear failure, continuous failure developed between Option C: Middle of the footing option B: Foundation and the ground surface Option C: Middle of the footing option Surface Option C: Middle of the footing option Surface	Option B:	Passive Earth pressure
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Option A: 80 cm Option B: 100 cm Option C: 75 cm Option D: 10 cm Q19. In general shear failure, continuous failure developed between Option A: Ground surface and footing Option B: Foundation and the ground surface Option C: Middle of the footing	Q18.	The standard penetration resistance is determined at a number of selected
Option B: 100 cm Option C: 75 cm Option D: 10 cm Q19. In general shear failure, continuous failure developed between Option A: Ground surface and footing Option B: Foundation and the ground surface Option C: Middle of the footing		points at interval of
Option C: 75 cm Option D: 10 cm Q19. In general shear failure, continuous failure developed between Option A: Ground surface and footing Option B: Foundation and the ground surface Option C: Middle of the footing	Option A:	80 cm
Option D: 10 cm Q19. In general shear failure, continuous failure developed between Option A: Ground surface and footing Option B: Foundation and the ground surface Option C: Middle of the footing	Option B:	100 cm
Q19. In general shear failure, continuous failure developed between Option A: Ground surface and footing Option B: Foundation and the ground surface Option C: Middle of the footing	Option C:	75 cm
Option A: Ground surface and footing Option B: Foundation and the ground surface Option C: Middle of the footing	Option D:	10 cm
Option A: Ground surface and footing Option B: Foundation and the ground surface Option C: Middle of the footing		
Option B: Foundation and the ground surface Option C: Middle of the footing	Q19.	In general shear failure, continuous failure developed between
Option C: Middle of the footing	Option A:	Ground surface and footing
	Option B:	Foundation and the ground surface
Option D: Edge of the footing and ground surface	Option C:	Middle of the footing
	Option D:	Edge of the footing and ground surface

Q20.	As per IS Code, maximum permissible differential settlement of clayey soil is
Option A:	25 mm
Option B:	40 mm
Option C:	65 mm
Option D:	100 mm
Q21.	The seating load for plate load test is
Option A:	2 kN/m ²
Option B:	3 kN/m^2
Option C:	4 kN/m ²
Option D:	7 kN/m^2
Q22.	The bearing capacity of a single pile in clay is mainly due to
Option A:	Friction
Option B:	Shear strength of soil
Option C:	Allowable load
Option D:	Ultimate load
Q23.	The load-carrying capacity of a pile CANNOT be determined by which of the
	following methods?
Option A:	Dynamic formulae
Option B:	Static formulae
Option C:	Pile load test
Option D:	Plate load test
Q24.	Which instrument is used to measuring displacement in pile load test load test
Option A:	Sand bag
Option B:	Dial Gauge
Option C:	Reaction Beam
Option D:	Hydraulic pump
Q25.	Settlement of Pile group in clay can be computed on assumption that
Option A:	The load is spread at an angle 30° with Horizontal
Option B:	The load is spread at an angle 90° with vertical
Option C:	The load is spread at an angle 30° with vertical
Option D:	The load is spread at an angle 60° with vertical