Program: Civil Engineering
Curriculum Scheme: Rev 2012
Examination: Second Year Semester IV
Course Code: CE-C 403 and Course Name: Structural Analysis I
Time: 1 hour
Max. Marks: 50

For the students:- All the Questions are compulsory and carry equal marks .

| Q1. | Top most part of an arch is called |
| :---: | :---: |
| Option A: | Sofit |
| Option B: | Crown |
| Option C: | Center |
| Option D: | Abutment |
|  |  |
| Q2. | Internal bending moment generated in a three hinged arch is always |
| Option A: | 0 |
| Option B: | Infinite |
| Option C: | Varies |
| Option D: | Non zero value but remains constant |
|  |  |
| Q3. | Internal shear force generated in a three hinged arch is always:- |
| Option A: | 0 |
| Option B: | Infinite |
| Option C: | Varies |
| Option D: | Non zero value but remains constant |
|  |  |
| Q4. | A three hinged Parabolic arch span of 30 m with rise of 5 m is loaded with a UDL of $10 \mathrm{kN} / \mathrm{m}$ of left half span. Calculate the horizontal thrust. |
| Option A: | 112.5 kN |
| Option B: | 75 kN |
| Option C: | 150 kN |
| Option D: | 50 kN |
|  |  |
| Q5. | Three hinged Parabolic arch loaded with a UDL W kN/m of span L m. BM at quarter span is. $\qquad$ |
| Option A: | WL^2/8 |
| Option B: | 0 |
| Option C: | WL^2/4 |
| Option D: | WL/4 |
|  |  |
| Q6. | Beam loaded with a UDL w kN/m with span L. Maximum deflection is |
| Option A: | ((wL^4)/(384EI)) |
| Option B: | ((wL^4)/(72EI)) |
| Option C: | ((wL^4)/(36EI)) |
| Option D: | ((wL^3)/(48EI)) |
|  |  |
| Q7. | Beam loaded with a UDL w kN/m with span L. Slope at midspan is |
| Option A: | ((wL^4)/(384EI)) |


| Option B: | ((wL^4)/(72EI)) |
| :---: | :---: |
| Option C: | 0 |
| Option D: | ((wL^3)/(48EI)) |
|  |  |
| Q8. | Macaulay's method is used to determine |
| Option A: | deflection |
| Option B: | strength |
| Option C: | toughness |
| Option D: | all of the above |
|  |  |
| Q9. | Macaulay's method equation is ......................... |
| Option A: | $\mathrm{EI}(\mathrm{d} 2 \mathrm{y} / \mathrm{dx} 2)=\mathrm{M}$ |
| Option B: | $\mathrm{EI}(\mathrm{d} 2 \mathrm{y} / \mathrm{dx} 2)=\mathrm{V}$ |
| Option C: | $\mathrm{EI}(\mathrm{d} 2 \mathrm{y} / \mathrm{dx} 2)=\mathrm{W}$ |
| Option D: | $\mathrm{EI}(\mathrm{d} 2 \mathrm{y} / \mathrm{dx} 2)=\mathrm{T}$ |
|  |  |
| Q10. | In this Conjugate beam method, shear in Conjugate beam is $\qquad$ in real beam |
| Option A: | Bending Moment |
| Option B: | Deflection |
| Option C: | Slope |
| Option D: | Shear Force |
|  |  |
| Q11. | In this Conjugate beam method, Bending moment in Conjugate beam is .........................in real beam |
| Option A: | Bending Moment |
| Option B: | Deflection |
| Option C: | Slope |
| Option D: | Shear Force |
|  |  |
| Q12. | Fixed joint in Real beam is replaced by .............. in Conjugate beam |
| Option A: | Fixed |
| Option B: | Free |
| Option C: | Hinged |
| Option D: | Roller |
|  |  |
| Q13. | Internal Hinge in Real beam is replaced by .............. in Conjugate beam |
| Option A: | Fixed |
| Option B: | Internal Hinge |
| Option C: | Hinge |
| Option D: | Free |
|  |  |
| Q14. | Which is the following term is associated with Unit load method |
| Option A: | $\mathrm{mM} / \mathrm{EI}$ |
| Option B: | M/mEI |
| Option C: | E/mMI |
| Option D: | I/EMm |
|  |  |


| Q15. | What will be equation used to calculate deflection in truss using Unit Load <br> Method |
| :---: | :--- |
| Option A: | $($ PKL $) /(\mathrm{AE})$ |
| Option B: | $(1 / 4)((\mathrm{PL}) /(\mathrm{AE}))$ |
| Option C: | $(1 / 2)((\mathrm{PLL}) /(\mathrm{AE}))$ |
| Option D: | $(1 / 3)((\mathrm{PLL}) /(\mathrm{AE}))$ |
|  |  |
| Q16. | In general tA/B implies |
| Option A: | Vertical deflection of tangent at B wrt that at A |
| Option B: | Vertical deflection of tangent at A wrt that at B |
| Option C: | Vertical deflection of extended tangent at B wrt tangent at A |
| Option D: | Vertical deflection of tangent at A wrt extended tangent at B |
|  |  |
| Q17. | The slenderness ratio is the ratio of |
| Option A: | Length of column to least radius of gyration |
| Option B: | Moment of inertia to area of cross-section |
| Option C: | Area of cross-section to moment of inertia |
| Option D: | Least radius of gyration to length of the column |
|  |  |
| Q18. | The Rankine formula holds good for |
| Option A: | Short column |
| Option B: | Long column |
| Option C: | Medium column |
| Option D: | Both short and long column |
|  |  |
| Q19. | For a column with Both end Fixed Leff=............L |
| Option A: | 1 |
| Option B: | 2 |
| Option C: | 4 |
| Option D: | $1 / 2$ |
|  |  |
| Q20. | For a column with one end Fixed and other enf free Leff=............L |
| Option A: | 4 |
| Option B: | 1 |
| Option C: | $1 / 2$ |
| Option D: | 2 |
| Q21. | in influence line diagrams (ILD) |
| Option A: | Points remain fixed, position of load changes |
| Option B: | Points change, position of loads remain fixed |
| Option C: | Both of them changes |
| Option D: | Neither of them changes |
| Option B: $:$ | $1 / 2$ |
| Option C: | 1 |
| Option D: | $1 / 4$ |


|  |  |  |
| :---: | :--- | :---: |
| Q23. | What will be the work done during additional application of dp1? <br> $\Delta=$ displacement caused when force is increased by a small amount <br> $\mathrm{P}=$ external force applied <br> $\mathrm{N}=$ internal force in the member force applied <br> $\mathrm{L}=$ length of member <br> $\mathrm{A}=$ cross-sectional area of member <br> $\mathrm{E}=$ Modulus of elasticity |  |
| Option A: | a) p1 d $\Delta 1+\mathrm{p} 2 \mathrm{~d} \Delta 2+\mathrm{dp} 1 \mathrm{~d} \Delta 1$ |  |
| Option B: | b) p1 d $\Delta 1+\mathrm{p} 2 \mathrm{~d} \Delta 2+1 / 2$ dp1d $\Delta 1$ |  |
| Option C: | c) p1 d $\Delta 1+1 / 2 \mathrm{p} 2 \mathrm{~d} \Delta 2+\mathrm{dp1d} \Delta 1$ |  |
| Option D: | d) $1 / 2 \mathrm{p} 1 \mathrm{~d} \Delta 1+\mathrm{p} 2 \mathrm{~d} \Delta 2+\mathrm{dp1d} \Delta 1$ |  |
|  |  |  |
| Q24. | The shape of cable under transverse uniformly distributed load is |  |
| Option A: | Parabolic |  |
| Option B: | Catenary |  |
| Option C: | Circular |  |
| Option D: | Triangular |  |
| Q25. | Unsymmetrical bending occurs due to |  |
| Option A: | The Beam cross section is unsymmetrical |  |
| Option B: | The shear Centre does not coincide with the neutral axis |  |
| Option C: | The Beam is subjected to trust in addition to bending moment |  |
| Option D: | The bending moment diagram is unsymmetrical |  |

